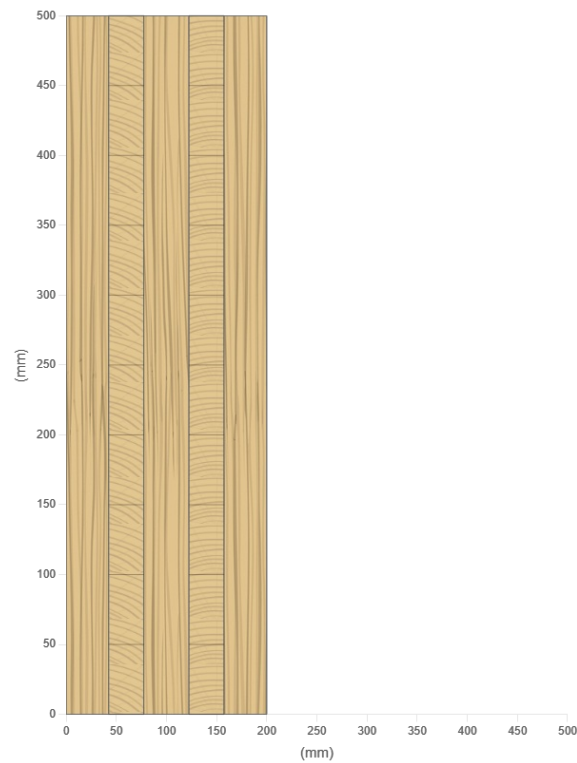
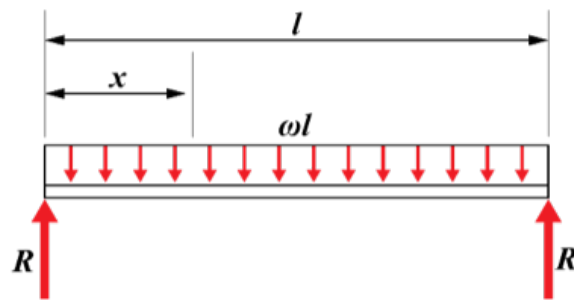


Supplier XLam	CLT Layup XL5/200	Orientation Primary Vertical	Method ProHolz		
Design Method Eurocode 5	Loading Code Eurocode 1	Load Combinations Default Load Combination	$f_{v,gross,k}$ (MPa) 2.5	$f_{v,net,k}$ (MPa) 5	$f_{t,k}$ (MPa) 1.5
<b>Loading Data</b> $G_{sdl}$ 2 (kN/m)	$Q$ 2 (kN/m)	$H_{beam}$ 0.5 (m)			
<b>Loading Conditions</b> Condition Simply Supported	Span 4.00 (m)	Transverse Direction Lamellas Width, $b_{90}$ (mm) 85 (mm)	Longitudinal Direction Lamellas Width, $b_0$ (mm) 50 (mm)		
<b>Design Data Eurocode 5</b> $\phi_{2,1}$ 0.3	$k_{sys}$ 1.08	$w_c$ 0.4	Service Class 1	Deflection Ratio (inst) 300	Deflection Ratio (net,fin) 300

**Output Summary**

Deflection	Bending	Shear
<b>OK</b>	<b>OK</b>	<b>OK</b>
39.24%	90.85%	22.71%



## Panel Properties

$H_{\text{beam}}$ (m)	0.5
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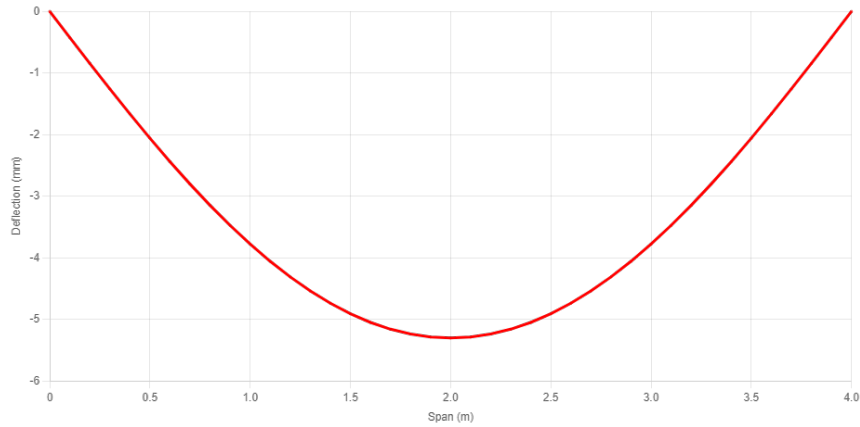
t	Orientation	Grade	E (MPa)	$f_b$ (MPa)	$\rho$ (kg/m <sup>3</sup> )	
t <sub>1</sub>	42.50	90	MGP10	0	17	500
t <sub>2</sub>	35.00	0	MGP6	6000	10	480
t <sub>3</sub>	45.00	90	MGP6	0	10	480
t <sub>4</sub>	35.00	0	MGP6	6000	10	480
t <sub>5</sub>	42.50	90	MGP10	0	17	500

## Calculate Section Modulus

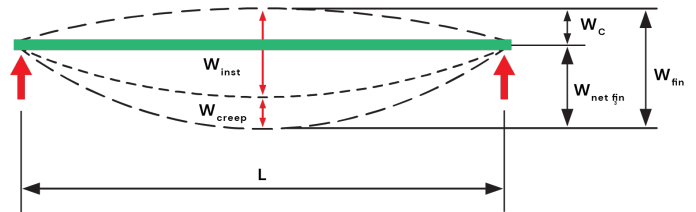
t	$d_{90}$ (mm)	$b_{\text{eff}} t_i^3 / 12$ (mm <sup>4</sup> )	$EI_{\text{eff}}$ (Nmm <sup>2</sup> )	$Z_{\text{eff}}$ (mm <sup>3</sup> )
t <sub>1</sub>	0.00	0.00	0.0e+0	0.0e+0
t <sub>2</sub>	35.00	85.00	3.6e+8	2.2e+12
t <sub>3</sub>	0.00	0.00	0.0e+0	0.0e+0
t <sub>4</sub>	35.00	85.00	3.6e+8	2.2e+12
t <sub>5</sub>	0.00	0.00	0.0e+0	0.0e+0

Bending Stiffness Properties of CLT		Primary
$EI_{\text{eff}}$ (Nmm <sup>2</sup> )	Bending Stiffness (assuming perfect connection between layers)	4.38e+12
$Z_{\text{eff}}$ (mm <sup>3</sup> )	Section modulus (using $EI_{\text{eff}}$ with perfect connection)	2.92e+6

Deflection

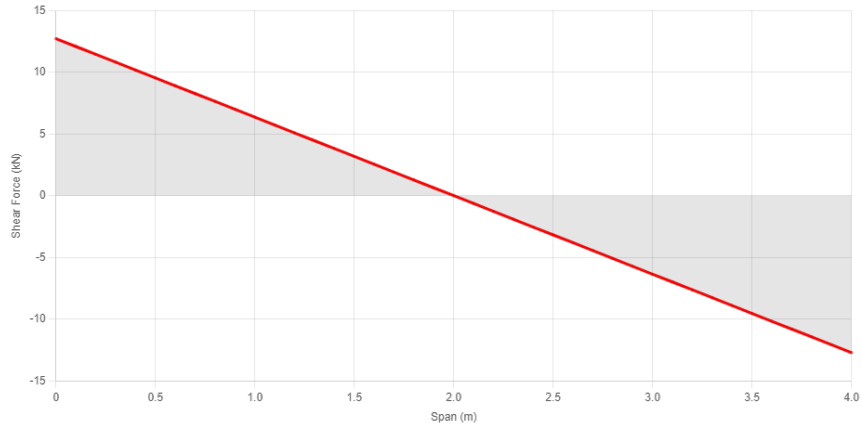


$G_{\text{beam sw}}$ (kN/m)	0.49	
$G_{\text{sdj}}$ (kN/m)	2.00	
$G_{\text{total}}$ (kN/m)	2.49	
$Q_{\text{total}}$ (kN/m)	2.00	
$\Psi_{2,1}$	0.30	
$k_{\text{def}}$	0.60	
$E_{\text{eff}}$ (Nmm <sup>2</sup> )	4.38e+12	Ambient conditions



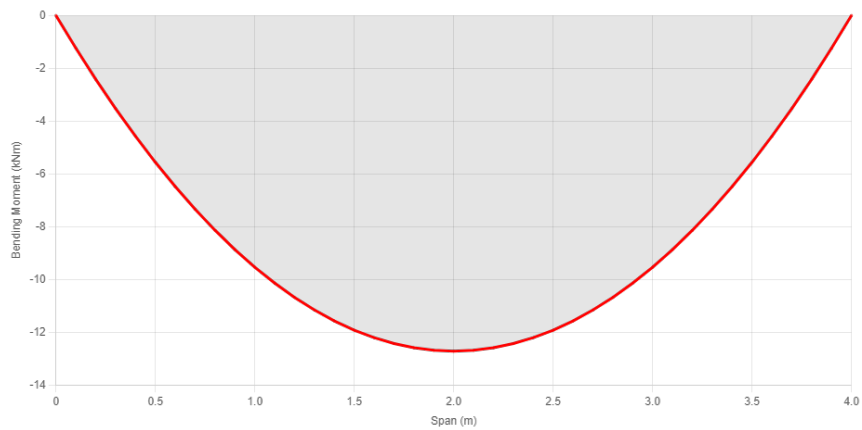
	w (kN/m)	Deflection (mm)	Limit (mm)	
$w_{\text{inst}}$	4.5	-3.4	13.33	25.6%
$w_{\text{creep}}$	1.9	-1.41		
$w_c$		0.40		
$w_{\text{fin}}$	6.3	-4.8	13.3	
$w_{\text{net,fin}}$		-5.23		39.2%

Shear Actions



	Ambient	Ambient
Load Combination	1.35G + 1.50Q	1.35G + 1.50Q + 0.90W
w (kN/m)	6.36	6.36
V* max	12.72	12.72

Bending Moment Actions



	Ambient	Ambient
Load Combination	1.35G + 1.50Q	1.35G + 1.50Q + 0.90W
M* <sub>+</sub> (kNm)	0.00	0.00
M* <sub>-</sub> (kNm)	-12.72	-12.72

**Bending Design**

$$\sigma_{m,z,d} = \frac{M_{z,d}}{W_{z,net}} \leq f_{m,d} = k_{mod} \frac{f_{m,k}}{\gamma_m}$$

$k_{sys}$   
1.08

$Z_{eff}$ (mm <sup>3</sup> )	2.92e+6	
$f_{m,k}$ (MPa)	10	Minimum of all layers assumed.

	1.35G + 1.50Q	1.35G + 1.50Q + 0.90W	
$k_{mod}$	0.60	0.90	
YM	1.25	1.25	
$f_{m,d}$ (kNm)	4.80	7.20	
$M_{z,d}^*$ (kNm)	12.72	12.72	
$\sigma_{m,y,d}$	4.36	4.36	
Ratio	91%	61%	91%

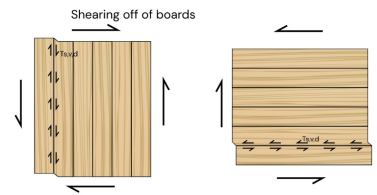
**Shear Design**

Mechanism 1: Shearing-off failure of the boards along a joint.

$$\tau_{v,net,d} \leq f_{v,net,d}$$

$$\tau_{v,net,d} = \frac{V_{net,d}}{A_{x,net}}$$

$$f'_{v,net,d} = k_{mod} \frac{f_{v,net,k}}{\gamma_m}$$



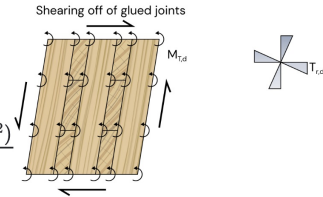
$f_{v,net,k}$ (MPa)	5.00	
$A_0$ (mm <sup>2</sup> )	3.50e+4	Along height of beam
$A_{90}$ (mm <sup>2</sup> )	5.20e+5	Along length of beam
$A_{x,net}$ (mm <sup>2</sup> )	2.33e+4	Minimum value taken

	1.35G + 1.50Q	1.35G + 1.50Q + 0.90W	
$k_{mod}$	0.60	0.90	
YM	1.25	1.25	
$f_{v,net,d}$ (MPa)	2.40	3.60	
$V_{max}$ (kN)	12.72	4.05	
$\tau_{net,d}$ (MPa)	0.55	0.17	
Ratio	22.71%	4.82%	23%

Mechanism 2: Shearing failure of the glued surface at the intersection of joints.

$$\tau_{T,d} \leq f_{v,T,d} \quad W_p = \frac{I_p}{\frac{a}{2}}$$

$$\tau_{T,d} = \frac{M_{T,d}}{n_t W_p}$$

$$f_{v,T,d} = k_{mod} \frac{f_{v,T,k}}{\gamma_m} \quad n_t = n_s \times n_f \quad I_p = \frac{bd(b^2+d^2)}{12}$$


$f_{t,k}$ (MPa)	1.50	Shear of glued surfaces (Mechanism 2)
$\Sigma n_k I_p$ (mm <sup>4</sup> )	1.38e+9	
$a_{des}$	50.00	Take largest board width in direction of load (most conservative)

	1.35G + 1.50Q	1.35G + 1.50Q + 0.90W	
$k_{mod}$	0.60	0.90	
YM	1.25	1.25	
$f_{v,T,d}$ (MPa)	0.72	1.08	
$M_{T,d}$ (kNm)	1.08	1.08	
$\tau_{T,d}$ (MPa)	0.02	0.02	
Ratio	3%	2%	3%

Mechanism 3: Shearing-off failure of the boards along a joint.

$$\tau_{v,gross,d} \leq f_{v,gross,d}$$

$$\tau_{v,gross,d} = \frac{V_{net,d}}{A_{gross}}$$

$$f_{v,gross,d} = k_{mod} \frac{f_{v,gross,k}}{\gamma_M}$$

$f_{v,gross,k}$ (MPa)	2.50	Shear of entire plate strength (Mechanism 3)
$A_{gross}$ (mm <sup>2</sup> )	1.00e+5	Along height of beam

	1.35G + 1.50Q	1.35G + 1.50Q + 0.90W	
$k_{mod}$	0.60	0.90	
YM	1.25	1.25	
$f_{v,gross,d}$ (MPa)	1.20	1.80	
$V_d$ (kN)	12.72	12.72	
$\tau_{gross,d}$ (MPa)	0.13	0.13	
Ratio	10.60%	7.07%	11%